

HYDRAULIC DESIGN CRITERIA

SHEETS 311-1 TO 311-5

TAINTER GATES ON SPILLWAY CRESTS

DISCHARGE COEFFICIENTS

1. Discharge through a partially open tainter gate mounted on a spillway crest can be computed using the basic orifice equation:

$$Q = CA \sqrt{2gH}$$

where,

Q = discharge in cfs

C = discharge coefficient

A = area of orifice opening in ft^2

H = head to the center of the orifice in ft.

The coefficient (C) in the above equation is primarily dependent upon the characteristics of the flow lines approaching and leaving the orifice. In turn, these flow lines are dependent upon the shape of the crest, the radius of the gate, and the location of the trunnion.

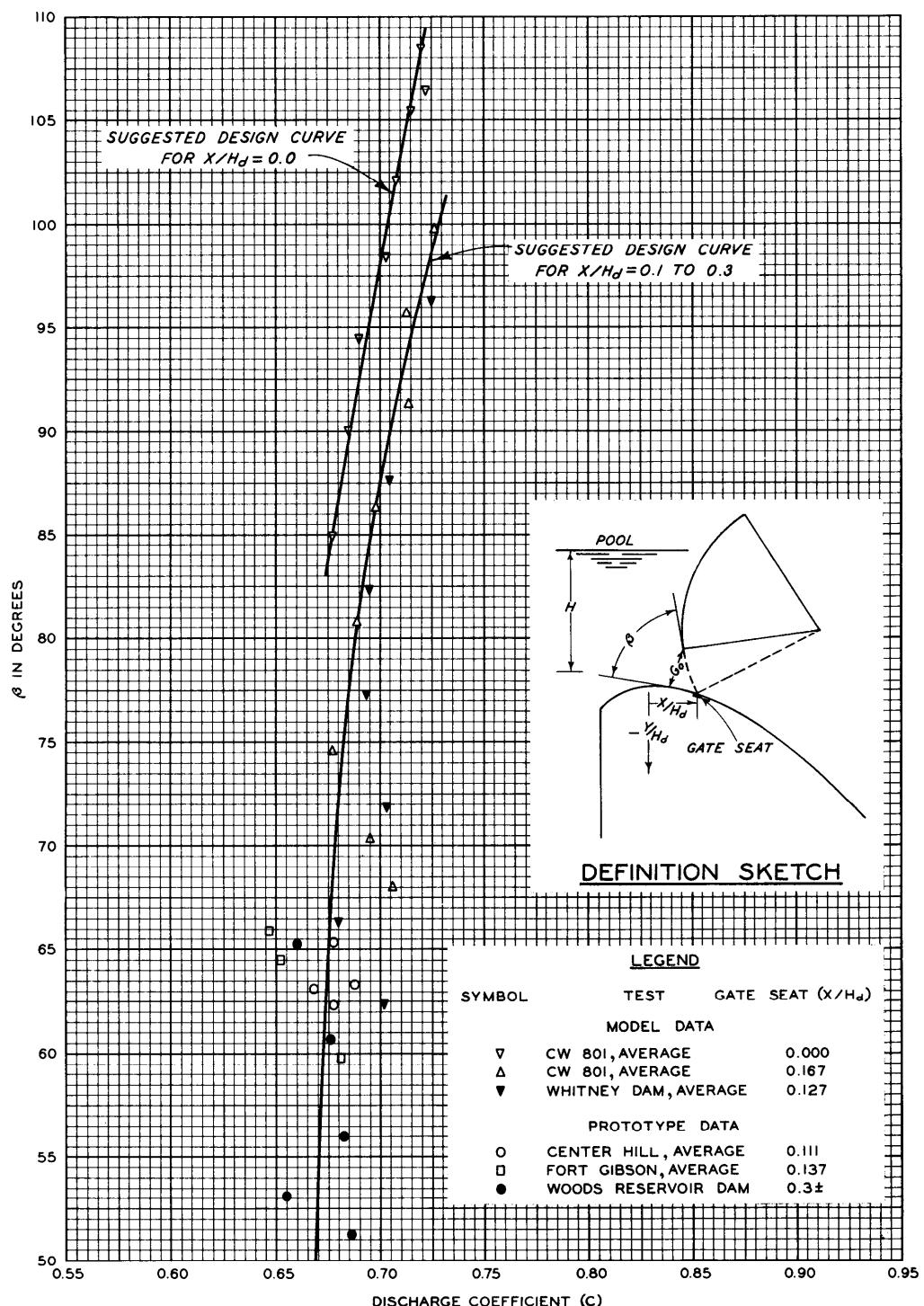
2. Discharge Coefficients. Chart 311-1 shows a plot of average discharge coefficients computed from model and prototype data for several crest shapes and tainter gate designs for nonsubmerged flow. Data shown are based principally on tests with three or more bays in operation. Discharge coefficients for a single bay would be lower because of side contractions although data are not presently available to evaluate this factor. On this chart, the discharge coefficient (C) is plotted as a function of the angle (β) formed by the tangent to the gate lip and the tangent to the crest curve at the nearest point of the crest curve. The net gate opening is considered to be the shortest distance from the gate lip to the crest curve. The angle is a function of the major geometric factors affecting the flow lines of the orifice discharge. One suggested design curve applies to tainter gates having gate seats located downstream from the crest axis. The other suggested design curve is based on tests with the gate seat located on the axis and indicates the effects of the masonry shape upstream from the crest axis.

3. Computation. Computation of discharge through a tainter gate mounted on a spillway crest is considerably complicated by the geometry involved in determining the net gate opening to be used in the orifice formula. The problem is simplified by fitting circular arcs to the crest

curve used in the design of spillways. Chart 311-2 illustrates the necessary computations to obtain the net gate opening and the angle β described in paragraph 2, for tainter gates mounted on spillway crests shaped to $X^{1.85} = -2 H_d^{0.85} Y$. All factors are expressed in terms of the design head (H_d). The method shown is applicable to other crest shapes. However, the accompanying design aids, Charts 311-3 and 311-4, apply only to standard crests.

4. To initiate the computations, Y_L/H_d values of the gate lip are assumed and corresponding values of X_L/H_d are computed (columns 1 to 6, Chart 311-2). These coordinates are then located on Chart 311-3 to determine the characteristics of a substitute arc. The substitute arc is then used to compute the net gate opening (columns 7 to 14). The point of intersection of the masonry line by the gate opening is determined by similar triangles (columns 14, 15, and 16). Design aid Chart 311-4 can be used to determine the Y_c/H_d coordinate of the gate opening and masonry line intersection (column 17), and also the slope of the masonry line (columns 18 and 19) which in turn combines with the slope of the gate lip tangent to form the angle β (column 20). If graphical methods are preferred to analytical methods, a large-scale layout will enable the head, net gate opening, and the angle β to be scaled so that the discharge can be computed with fair accuracy.

5. Chart 311-5 is a sample computation of the steps involved in the development of a rating curve for a partially open tainter gate. The final computations are dimensional and are believed accurate to within ± 2 per cent, for gate opening-head ratios (G_o/H) less than 0.6.



FORMULA

$$Q = C G_o B \sqrt{2gH}$$

WHERE:

G_o = NET GATE OPENING
 B = GATE WIDTH
 H = HEAD TO CENTER OF GATE OPENING

Tainter Gates on Spillway Crests

Discharge Coefficients

Hydraulic Design Chart 3II-1

COMPUTATION SHEET

GATE OPENINGS AND ANGLE β

JOB CW804 PROJECT JOHN DOE DAM
 SUBJECT SPILLWAY DISCHARGE
 COMPUTED BY AAMS DATE 8-24-54
 CHECKED BY HAB DATE 8-26-54

GIVEN

DESIGN HEAD (H_d) = 37.0 FT.
 RADIUS OF GATE (R_g) 0.831 H_d .
 TRUNNION COORDINATES (X_t, Y_t).
 $X_t = 0.907 H_d$, $Y_t = 0.324 H_d$.

DEFINITIONS

GATE LIP COORDINATES (X_L, Y_L).

SPILLWAY CREST COORDINATES (X_c, Y_c).

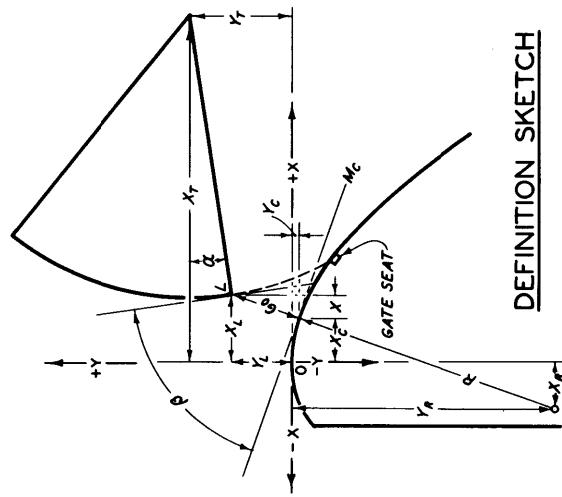
SLOPE OF TANGENT TO CREST (M_c), NEGATIVE
 WHEN DOWNSTREAM FROM CREST.

SHORTEST DISTANCE FROM GATE LIP TO CREST (G_0).

DEFINITIONS (CONT)

α IS THE ANGLE BETWEEN A LINE CONNECTING THE
 GATE LIP AND THE TRUNNION CENTER, AND A
 HORIZONTAL LINE THROUGH THE TRUNNION, CON-
 SIDERED POSITIVE AND NEGATIVE WHEN THE GATE
 LIP IS ABOVE AND BELOW THE TRUNNION, RESPEC-
 TIVELY.

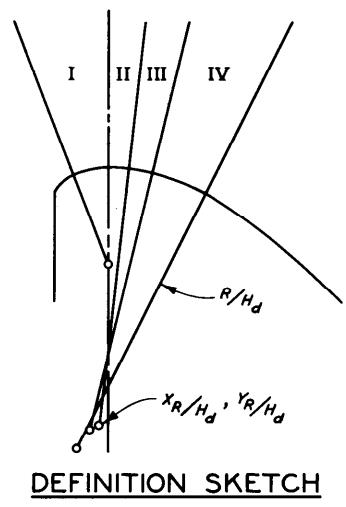
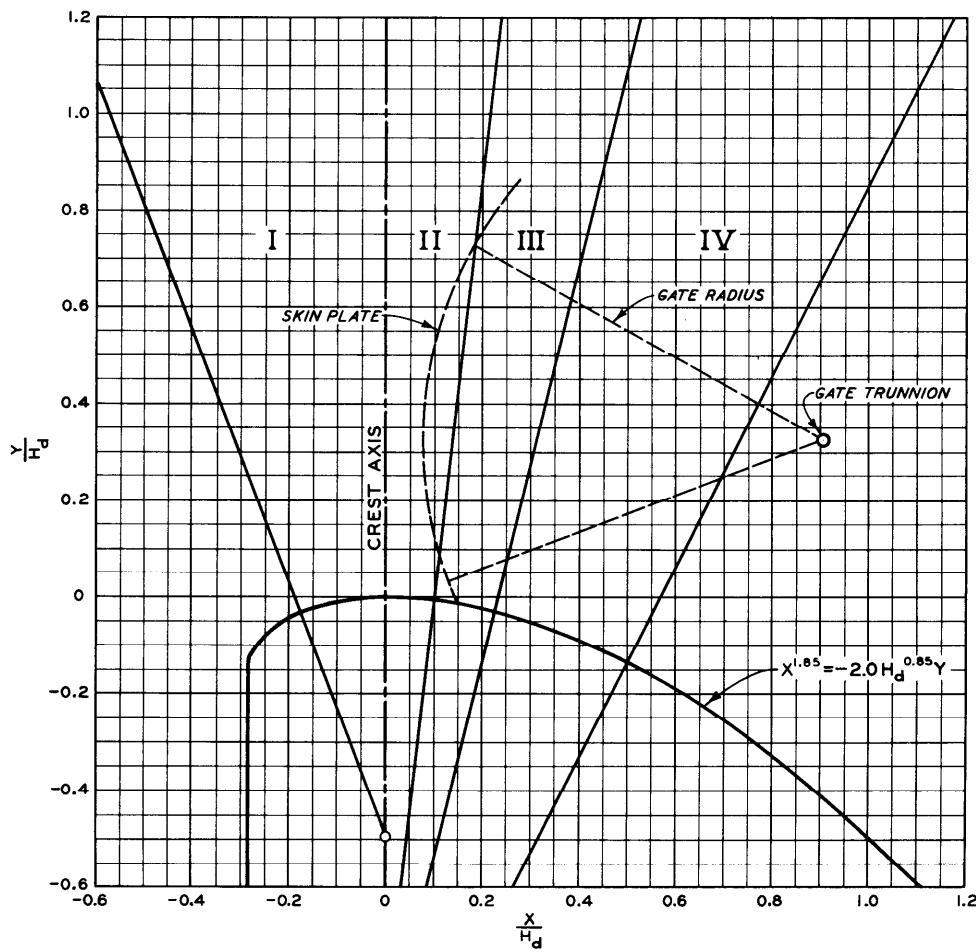
NOTE: ALL DIMENSIONS USED IN COMPUTATIONS ARE
 IN TERMS OF DESIGN HEAD (H_d).



DEFINITION SKETCH

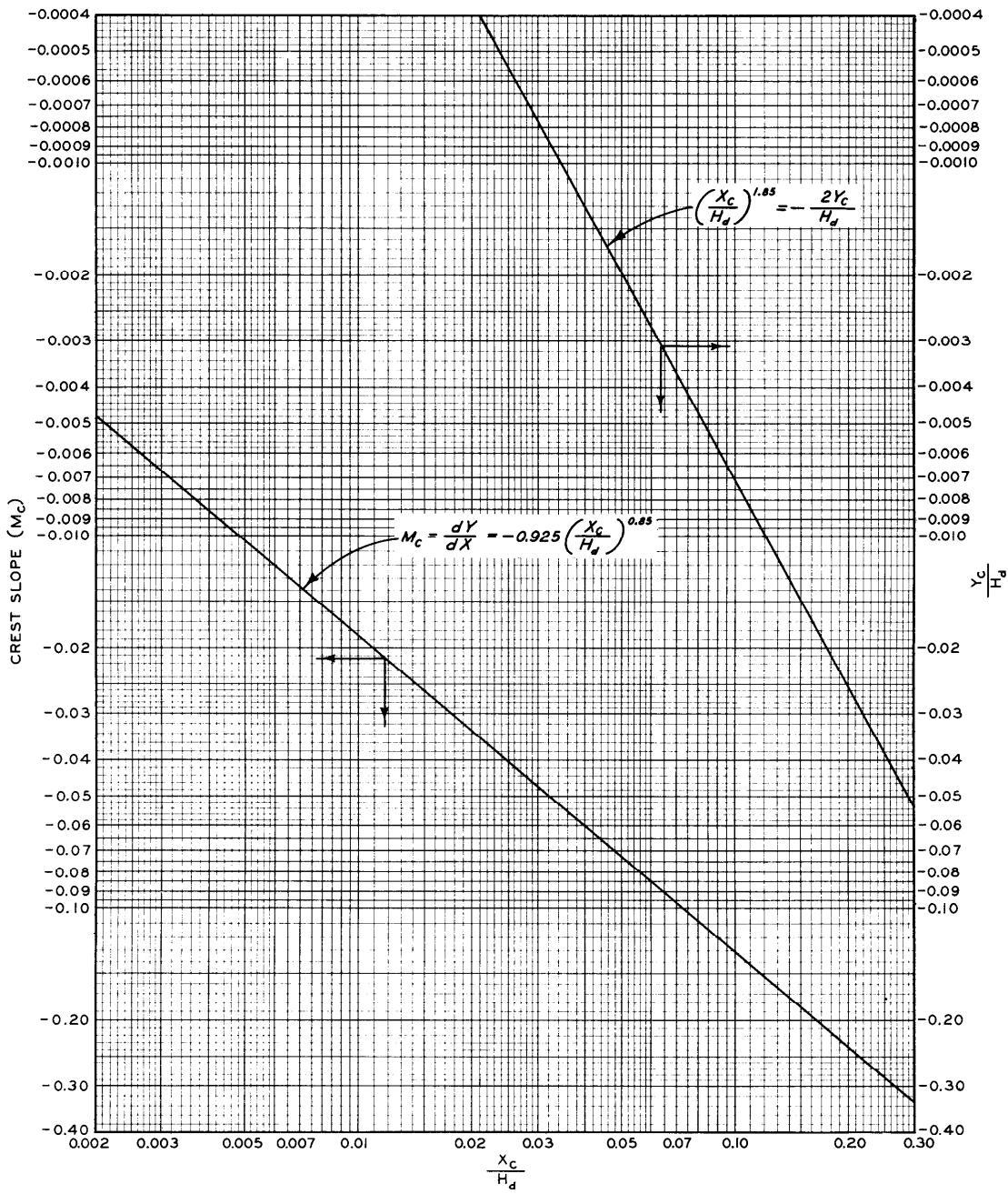
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)
Y_L	$Y_t - Y_L$	$\sin \alpha$	α	$R_g \cos \alpha$	$R_g \sin \alpha$	X_L	FROM CHART 3II-3	$X_L - X_R$	$Y_L - Y_R$	$R + G_0$	G_0	X	x	x	FROM CHART 3II-4	$TAN^{-1} M_c$	$90^\circ + \tan^{-1} M_c + \alpha$		
	$(2) \div R_g$	$(2) \div R_g$	DEGREES	$0.831 \cos \alpha$	$0.831 \sin \alpha$	$X_T - (5)$	CLASS	X_R	Y_R	R	$(6) - (8)$	$(1) - (9)$	$(1) - (9)$	$(1) - (10)$	$(1) - (12)^2 / 2$	$(1) - (12)^2 / 2$	$90^\circ + (19) + (4)$	DEGREES	
0.100	0.224	0.270	-15.67	0.800	0.107	II	-0.050	-1.329	1.330	0.157	1.4229	1.437	0.107	0.012	0.095	-0.0085	-0.125	-7.13	67.20
0.200	0.124	0.149	-8.57	0.821	0.086	II	-0.050	-1.329	1.330	0.136	1.329	1.535	0.205	0.018	0.068	-0.0035	-0.094	-5.35	76.06
0.300	0.024	0.029	-1.66	0.830	0.077	II	-0.050	-1.329	1.330	0.127	1.629	1.634	0.304	0.024	0.053	-0.0022	-0.076	-4.36	83.98
0.400	0.076	-0.091	+5.22	0.829	0.078	II	-0.050	-1.329	1.330	0.128	1.729	1.733	0.403	0.030	0.048	-0.0018	-0.070	-4.02	91.20

Tainter Gates on Spillway Crests
 Sample Geometric Computation
 Hydraulic Design Chart 3II-2



CLASS	R/H_d	X_R/H_d	Y_R/H_d
I	0.500	0.000	-0.500
II	1.330	-0.050	-1.329
III	1.359	-0.100	-1.351
IV	1.472	-0.164	-1.452

TANTER GATES ON
SPILLWAY CRESTS
GEOMETRIC FACTORS
HYDRAULIC DESIGN CHART 3II-3



Tainter Gates on
Spillway Crests
Crest Coordinates and
Slope Function

Hydraulic Design Chart 3II-4

WATERWAYS EXPERIMENT STATION
COMPUTATION SHEET

JOB CW804 PROJECT JOHN DOE DAM SUBJECT SPILLWAY DISCHARGE
 COMPUTATIONS COORDINATES FOR RATING CURVE (POOL VS DISCHARGE FOR VARIOUS GATE OPENINGS)
 COMPUTED BY AAMG DATE 8-25-54 CHECKED BY RRW DATE 8-27-54

GIVEN

DESIGN HEAD (H_d) = 37.0 FT
 GATE WIDTH (B) = 42.0 FT
 CREST ELEV = 288.0 FT

FORMULAS

$$Q = C g_0 B \sqrt{2gH}$$

$$H = \text{POOL ELEV} - 0.5 [\text{ELEV } Y_c + \text{ELEV } Y_L]$$

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)
β^* DEGREES	C^{**}	G_0/H_d^*	G_0	Y_L/H_d^*	Y_L	Y_c/H_d^*	Y_c	$ELEV\ Y_L =$ $288 + Y_L$ FT	$ELEV\ Y_c =$ $288 + Y_c$ FT	$(9) + (10)$ 2	POOL	$(12) - (11)$ FT	$H^{1/2}$	Q CFS
67.20	0.676	0.107	3.96	0.100	3.70	-0.0065	-0.24	291.70	287.76	289.73	300	10.27	3.20	2,900
76.06	0.683	0.205	7.59	0.200	7.40	-0.0035	-0.13	295.40	287.87	291.64	315	25.27	5.03	4,500
83.98	0.694	0.304	11.25	0.300	11.10	-0.0022	-0.08	299.10	287.92	293.51	310	35.27	5.94	5,400
91.20	0.707	0.403	14.91	0.400	14.80	-0.0018	-0.07	302.80	287.93	295.37	315	18.36	4.28	7,500
											325	23.36	4.83	8,400
											325	33.36	5.78	10,100
											325	31.49	4.64	12,200
											320	19.63	5.61	14,800
											320	24.63	4.96	15,800
											325	29.63	5.44	17,600
														19,300

Tainter Gates on Spillway Crests
Sample Discharge Computations

Hydraulic Design Chart 3II-5

* FROM HYDRAULIC DESIGN CHART 3II-2
 ** FROM HYDRAULIC DESIGN CHART 3II-1